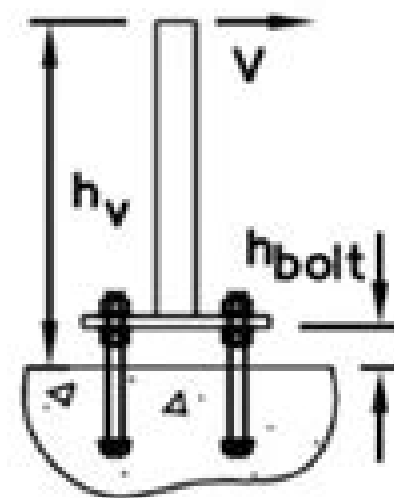
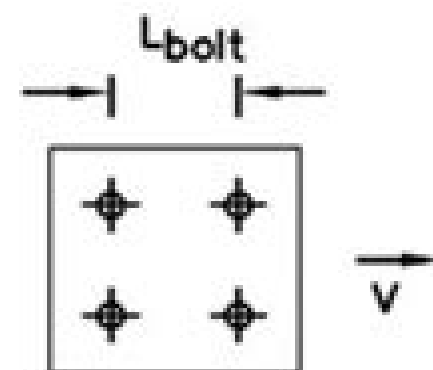


Check Base Plate Bolts in Bending and Shear

$\Phi_{nom} =$	0.5	in = nominal bolt diameter
$K_{bolt} =$	0.417	in = min root bolt diameter
$F_y =$	36	ksi = bolt yield strength
$N =$	4	= number of bolts
$h_{bolt} =$	2	in = height between TOConc and BOSteel
$L_{bolt} =$	5	in = distance between bolts, parallel to V
$V =$	200	lb = shear applied to bolts
$h_v =$	42	in = height above TOC where V is applied
$h_{bolt} \leq 2\Phi_{nom}?$	FALSE	therefore, clear span is long enough to develop significant moment
$M_{bolt} =$	50	lb * in = $V * h_{bolt} / 2 / N$ = moment on a bolt (assumed double curvature)
$V_{bolt} =$	50	lb = V / N = shear on a bolt
$T_{bolt} =$	800	lb = $V * (h_v - h_{bolt}) * 2 / L_{bolt} / N$ = tension on a bolt
$z_{bolt} =$	0.0121	in ³ = $K_{bolt}^3 / 6$ = plastic section modulu
$\Omega =$	1.67	= failure mode safety factor
$M_{AL} =$	261	lb*in = $F_y * z_{bolt} / \Omega$ = allowable tension
$V_{AL} =$	2,646	lb = allowable shear
$T_{AL} =$	3,195	lb = allowable tension
$M_{DCR} =$	0.192	= M_{bolt} / M_{AL} = moment DCR
$V_{DCR} =$	0.019	= V_{bolt} / V_{AL} = shear DCR
$T_{DCR} =$	0.250	= T_{bolt} / T_{AL} = tension DCR
$DCR =$	0.443	= $((M_{DCR} + T_{DCR})^2 + V_{DCR}^2)^{0.5} \leq 1.00 \therefore OK$



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**Mark D. Denavit, Ali Nassiri, Mustafa
Mahamid, Martin Vild, Halil
Sezen, Frantisek Wald, IDEA StatiCa**

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