

4.3.2.5 Round tubes

β should be found as follows, with no distinction between axial compression and bending:

$$\beta = 3 (Dt)^{75}$$

where

D is the diameter to mid-metal;

t is the thickness.

4.3.3 Section classification

4.3.3.1 General

The procedure is to classify the individual elements comprising the section, except for any element stressed wholly in tension. The classification of the section is then taken as that for the least favourable element. Individual elements are classified in accordance with 4.3.3.4 or 4.3.3.5.

4.3.3.2 Sections: beams and struts

For the section of a beam (moment resistance) or of a strut (axial force resistance) the following classifications apply.

(a) Moment resistance:

- (1) fully compact: local buckling can be ignored;
- (2) semi-compact: the section can develop a moment equal to p_o times the elastic section modulus;
- (3) slender: the moment resistance is reduced by premature local buckling at an extreme fibre stress below p_o .

(b) Axial compression resistance:

- (1) compact: local buckling can be ignored;
- (2) slender: local buckling lowers the resistance.

4.3.3.3 Sections subject to combined actions

For the classification of sections required to carry biaxial bending, or simultaneous bending with axial force, see 4.8.2.1.

4.3.3.4 Element classification

The classification of an individual element depends on the value of β (see 4.3.2) as follows:

(a) elements in beams (moment resistance):

$\beta \leq \beta_1$ fully compact

$\beta_1 < \beta \leq \beta_o$ semi-compact

$\beta > \beta_o$ slender

(b) elements in struts (axial resistance):

$\beta \leq \beta_o$ compact

$\beta > \beta_o$ slender

where β_o and β_1 are as given in table 4.3.

Table 4.3 Limiting values of β

Elements	β_o		β_1	
	Unwelded	Welded	Unwelded	Welded
Outstand elements	7 ϵ	6 ϵ	6 ϵ	5 ϵ
Internal elements	22 ϵ	18 ϵ	18 ϵ	15 ϵ

NOTE 1. The quantity ϵ should generally be taken as follows (except for certain flange elements in beams, see 4.3.3.5):

$$\epsilon = (250/p_o)^{75}$$

where

p_o is the limiting stress (in N/mm²) (see tables 4.1 and 4.2).

NOTE 2. An element is considered as welded if it contains welding at an edge or at any point in its width. When the stability of a particular cross-section of a member is evaluated, however, it is permissible to consider an element as unwelded if it contains no welding at that section, even though it is welded elsewhere along its length.

NOTE 3. In a welded element the classification is independent of the extent of the HAZ.

4.3.3.5 Understressed flange elements

A more favourable classification may if desired be taken for flange elements in members under bending, or bending with axial force, that are both:

- a) parallel to the axis of bending; and
- b) less highly stressed than the most severely stressed fibres in the section.

For these it is permissible, in using table 4.3, to take a modified value of ϵ as follows:

$$\epsilon = (250y_1/p_o y_2)^{75}$$

where y_1 and y_2 are the distances from the neutral axis of the gross section to the most severely stressed fibres and to the element respectively. They should generally be measured from the elastic neutral axis, although in checking whether a section is fully compact it is permissible to use the plastic one.

4.3.4 Local buckling

4.3.4.1 General

The possibility of local buckling in members classified as slender is generally allowed for by replacing the true section by an effective one. The effective section is obtained by employing a local buckling coefficient k_L to factor down the thickness, this being applied to any uniform thickness slender element that is wholly or partly in compression. Elements that are not uniform in thickness require special study.

4.3.4.2 Determination of k_L

The coefficient k_L , which is found separately for different elements of the section, is read from the appropriate curve in figure 4.5 selected in accordance with table 4.4. In order to select the correct curve the value of β/ϵ should be determined as follows:

β is found as in 4.3.2;

$\epsilon = (250/p_o)^{75}$ generally (but see note 3 of 4.5.2.3 for beam compression flanges);

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M. J. Ryall, G. A. R. Parke, J. E. Harding



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