

		Table 4-1 (continued) Available Strength in Axial Compression, kips W-Shapes												 W14-I	
Shape		W14-													
Lb/Tb		140		130		120		100		80		60			
Design		$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c	$\phi_c P_n$	P_n/Ω_c		
length, L_b (ft), with respect to least radius of gyration, r_y	0	1290	1080	1160	970	1000	830	950	790	840	700	820	680	1160	970
	5	1290	1080	1130	940	1000	830	920	760	840	700	820	680	1160	970
	7	1240	1030	1120	930	1000	830	920	760	840	700	820	680	1160	970
	8	1230	1020	1110	920	1000	830	910	750	840	700	820	680	1160	970
	9	1210	1000	1090	900	1000	830	900	740	840	700	820	680	1160	970
	10	1200	990	1080	890	1000	830	890	730	840	700	820	680	1160	970
	11	1180	970	1060	870	1000	830	870	710	840	700	820	680	1160	970
	12	1160	950	1040	850	1000	830	850	690	840	700	820	680	1160	970
	13	1140	930	1020	830	1000	830	840	670	840	700	820	680	1160	970
	14	1120	910	1000	810	1000	830	820	650	840	700	820	680	1160	970
	15	1100	890	980	790	1000	830	800	630	840	700	820	680	1160	970
	16	1080	870	960	770	1000	830	780	610	840	700	820	680	1160	970
	17	1060	850	940	750	1000	830	760	590	840	700	820	680	1160	970
	18	1030	830	910	730	1000	830	730	570	840	700	820	680	1160	970
	19	1010	810	890	710	1000	830	710	550	840	700	820	680	1160	970
	20	990	790	870	690	1000	830	690	530	840	700	820	680	1160	970
	22	930	730	810	630	1000	830	630	470	840	700	820	680	1160	970
	24	870	670	750	570	1000	830	570	410	840	700	820	680	1160	970
	26	810	610	690	510	1000	830	510	350	840	700	820	680	1160	970
	28	750	550	630	450	1000	830	450	290	840	700	820	680	1160	970
	30	700	500	580	400	1000	830	400	240	840	700	820	680	1160	970

From AISC *Manual*/Table 4-1, the available strength of a W14-90 with an effective length of 18 ft is:

LRFD	ASD
$\phi_c P_n = 929 \text{ kips} > 840 \text{ kips}$	$\frac{P_n}{\Omega_c} = 618 \text{ kips} > 560 \text{ kips}$
a.k.	a.k.

The available compressive strength is governed by the x - x axis flexural buckling limit state.

The available strengths of the columns described in Examples E.1A and E.1B are easily selected directly from the AISC *Manual*/Tables. The available strengths can also be verified by hand calculations, as shown in the following Examples E.1C and E.1D.

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R Barnett



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as specified by AISC Specifications Thus the use of the AISC Steel Construction Manual is a must in teaching this course In the second semester Chapters 9 through 13 should be covered At the undergraduate level Chapters 11 through 13 should be used on a limited basis leaving the student more time to concentrate on composite construction and built up girders

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